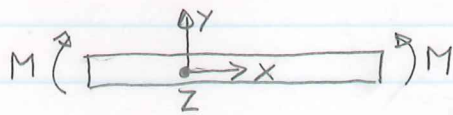


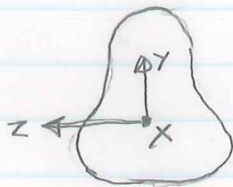
15. Pure bending with a symmetric cross-section

This section deals with bending under constant bending moment. There is no resultant shear or axial force on a cross-section and no resultant torque. The material is linearly elastic, homogeneous and isotropic.



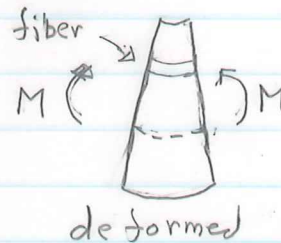
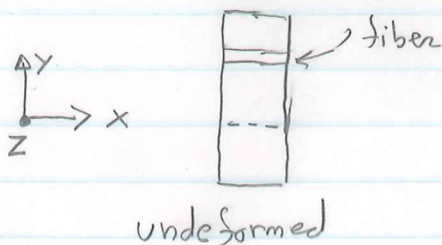
x is the axis along the beam,
 y - z is the cross-sectional plane.

The bending moment M is shown positive.



← This shows a cross-section. Symmetry exists about the x - y plane. The bending moment is about the z axis.

Examine a length of the beam.



The cross-sections rotate and remain plane.

Under positive M , the fibers in the top portion of the beam get shorter and those in the bottom portion get longer. Because the cross-sections remain plane, ϵ_x is a linear function of y . Along the dashed line, ϵ_x is zero. This line is called the neutral axis and is where the z axis is located. (The x - z plane is the neutral plane.) For positive M , ϵ_x is compressive for positive y and tensile for negative y .

Every fiber in a beam under constant bending moment deforms into the arc of a circle. Let ρ be the radius of curvature of the fibers in the deformed x - z plane, where

$\epsilon_x = 0$. Also, L = length of beam
and θ = arc angle.
We have

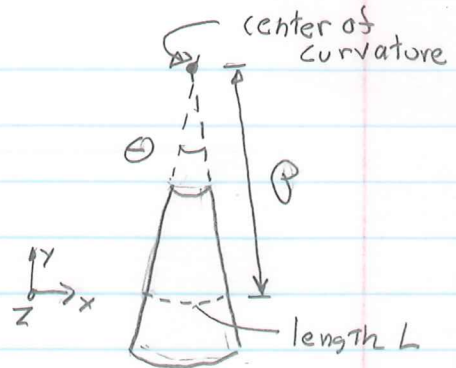
$$\epsilon_x = \frac{L - (\rho - y)\theta}{L}$$

$$= -\frac{y}{\rho}$$

Since $\theta = \frac{L}{\rho}$.

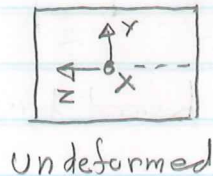


undeformed

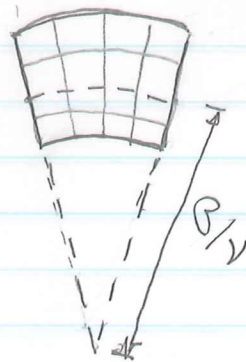


deformed

It can be shown that pure bending produces only σ_x ; the other stress components are all zero. In addition to ϵ_x , the other strains are $\epsilon_y = \epsilon_z = -\nu \epsilon_x$ and zero for the shear strains. Because of the presence of ϵ_y and ϵ_z , the cross-section changes shape during bending. For example, consider a rectangular cross-section.

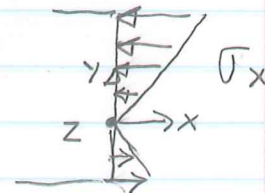


undeformed



deformed
(anti-clastic curvature)

Over the cross-section, $\sigma_x = E \epsilon_x = -E \frac{y}{\rho}$. For positive moment, compressive stresses (and strains) occur above the neutral axis and tensile stresses (and strains) occur below. The variation is linear with y .



Since the resultant axial force (x direction) on a cross-section is zero:

$$\int_{c-s} \sigma_x dA = -\frac{E}{\rho} \int_{c-s} y dA = 0,$$

which implies the neutral axis (z axis) is a centroidal axis for a cross-section.

We would like to relate the bending moment M to the stresses σ_x .

$$M = \int_{C-S} -y \sigma_x dA = \frac{E}{\rho} \int_{C-S} y^2 dA = \frac{EI}{\rho}$$

where $I = \int_{C-S} y^2 dA$ is the moment of inertia of the cross-section. (y measured from the neutral axis) In the above equation, ρ is replaced by

$$\rho = -\frac{y}{\epsilon_x} = -\frac{E y}{\sigma_x} \quad \text{to give}$$

$$\sigma_x = -\frac{M y}{I}$$

Define $c = |y_{\max}|$, the distance from the neutral axis to the top or bottom fiber, whichever is greater.

Then

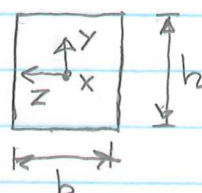
$$\sigma_{\max} = \frac{M c}{I}, \quad \text{the maximum normal stress, whether tension or compression.}$$

This last equation is sometimes written as

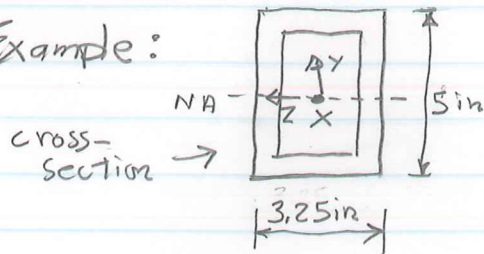
$$\sigma_{\max} = \frac{M}{S} \quad \text{where } S \text{ is the section modulus} = \frac{I}{c}.$$

For example, for a rectangular cross-section,

$$S = \frac{\frac{1}{12} b h^3}{\frac{h}{2}} = \frac{1}{6} b h^2$$



Example:



wall thickness = 0.25 in

$$E = 10.6 \times 10^6 \text{ psi}$$

$$C = 2.5 \text{ in}$$

$$I = \frac{1}{12} (5 \text{ in})^3 \cdot 3.25 \text{ in} - \frac{1}{12} (4.5 \text{ in})^3 \cdot 2.75 \text{ in}$$

$$= 12.97 \text{ in}^4$$

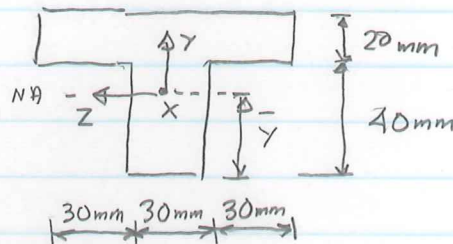
Find M_{allow} for $\sigma_{allow} = 20 \text{ ksi}$

$$M_{allow} = \sigma_{allow} \cdot I / c = 103.8 \text{ kip-in}$$

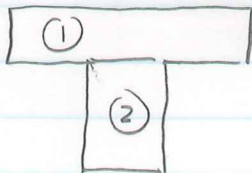
Find ϕ when $M = M_{allow}$

$$\phi = EI / M = 110.4 \text{ ft}$$

Example:

Find $\bar{Y}$ (location of neutral axis) I , c , S

distance to centroid of each piece



piece	A_i	$\bar{Y}_i$	$A_i \bar{Y}_i$
1	1800 mm^2	50 mm	$90,000 \text{ mm}^3$
2	1200 mm^2	20 mm	$24,000 \text{ mm}^3$

$$\bar{Y} = \frac{\sum A_i \bar{Y}_i}{\sum A_i} = \frac{114,000 \text{ mm}^3}{3,000 \text{ mm}^2} = 38 \text{ mm}$$

$$c = \max(38 \text{ mm}, 60 \text{ mm} - 38 \text{ mm}) = 38 \text{ mm}$$

$$I = \frac{1}{12} 90 \cdot 20^3 + 90 \cdot 20 \cdot (50 - 38)^2 + \frac{1}{12} 30 \cdot 40^3 + 30 \cdot 40 \cdot (20 - 38)^2$$

$$= 868,000 \text{ mm}^4$$

all units mm

$$S = I / c = 22,842 \text{ mm}^3$$

($\sigma_{max} = M / S$ will give maximum stress at bottom fiber.)